



Contribution of natural asphalts to the implementation of HMA mix for cold climate

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- 1. Modification of petroleum road bitumen by the addition of natural asphalts
- 2. Characteristics and low temperature behaviour of Selenizza SLN and natural asphalt modified binders
- 2. Mix design examples used in countries with vast temperature fluctuations. Selection criteria
- 4. Natural bitumen: An environmentally friendly product
- 5. Conclusions



Characterisation of different natural asphalts

- A study was carried out by the University of Rome "LA SAPIENZA" to characterize natural bitumen and evaluate their contribution to the modification of straight-run bitumen
- The aim of this research work was to characterize some of the natural asphalts, most diffused commercially and to evaluate their efficiency as modifiers
- Three natural asphalts were selected:

| Natural asphalt | Bitumen content (%) | Asphaltènes content(%) | Penetration (à 25°C,1/10 mm) | R&B (°C) |
|--------------------|------------------------|------------------------|---------------------------------|-------------|
| Gilsonite | > 99 | 70 | 0 | 160-170 |
| Selenizza | 85-90 | 42* | 0 | 115 |
| Trinidad | 53-55 | 33-37 | 1 - 4 | 93-98 |

- An Iranian **Straight Run bitumen** (Gach Saran) with penetration **80-100**, was **added with each** of the three types of natural asphalts :
 - by the percentage of 10%
 - at a minimum temperature of 150 180 °C
- Two techniques have been used:
 - Dynamic rheological analysis & Modulated Differential Scanning Calorimetry (MDSC)

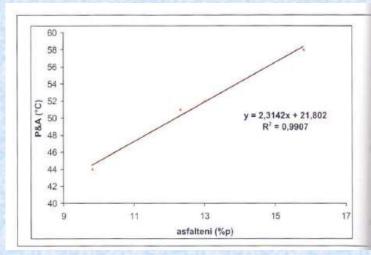


Characterisation of different natural asphalts

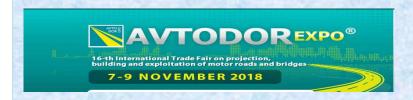
Effect on Penetration and Softening Point

As **expected**, for the three cases, the resulting modified bitumen was characterized by **higher softening point** (R&B temperatures) and **lower penetration values**, compared to the original standard bitumen, due to the presence of **high percentages of asphaltenes** content in the natural asphalts

| Type of bitumen | Penetration at 25" | R&B Temperature | Asphaltenes content |
|------------------|--------------------|-----------------|---------------------|
| | (1/10 mm) | 'c | (%) |
| Original bitumen | 96 | 44 | 9,8 |
| +10% Gilsonite | 38 | 58 | 15,8 |
| +10% Selenizza | 67 | 52 | 13,0 |
| +10% Trinidad | 78 | 51 | 12,3 |

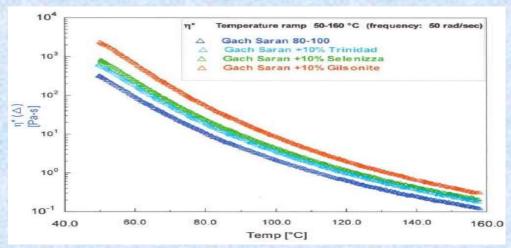


Relationship between aspaltene content and R&B



Effect on viscoelastic properties at high temperatures

For medium and high temperatures (50 - 160°C), the rheological behavior whose softening point represent the lower limit, is not a function of the modifier quality and depends exclusively on the asphaltenes content



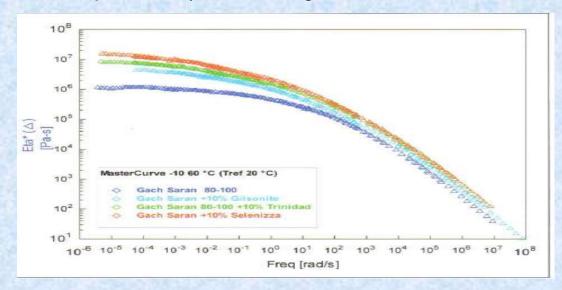
The viscosity values increase, the viscosity curves shift upwards, their shape and the slope remain unchanged and parallel for all sample types. The modifiers don't affect the internal interactions between the asphaltene components in the modified bitumen, which is a typical phenomenon for the compatible additives



Effect on viscoelastic properties at low temperatures

For the low temperatures (10 - 60°C), the rheological modifications seem complex and are

differentiated

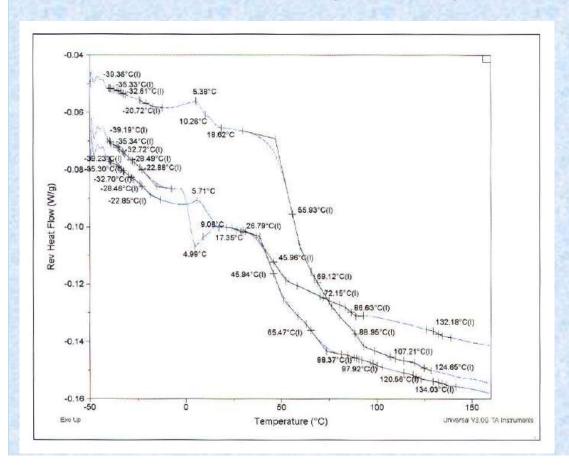


Master curves $η^*$, G', G'' = f(ω) drawn under reference temperature 20°C & temperature interval [-10 ÷ 60 °C]. The viscoelastic behavior and the ductility of the modified samples are impacted by the quality of the natural bitumen (bituminous+inorganic component). At T=20°, inversion of the zero shear viscosity $η_0$ (GS) < $η_0$ (Gil) < $η_0$ (Trid < $η_0$ (Sln)



Characterisation of different natural asphalts

Modulated Differential Scanning calorimetry MDSC



The **samples** (7 – 10 mg), were **subjected to a modulated heating ramp** resulting from a sinusoidal temperature ripple overlaid on a linear temperature ramp:

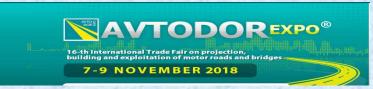
 $dQ/dt = C_p β + f(T, t)$ Temperature range: [-50 °C, + 160 °C]

For the bitumen, the reversing curve $\approx 1/C_p$, is more indicative: vitreous transitions, fusions

Trinidad & Selenizza: affect the lower limit of the softening range of the straight run bitumen (+55,8 °C → 45,9 °C) due to the presence of different maltenic phases (of lower molar mass), which soften at lower temperatures. The asphaltenic phases, result to behave independently. A dilution effect of the original bitumen is obtained

Gilsonite, does not act as a diluent, but expands the softening range to higher temperatures

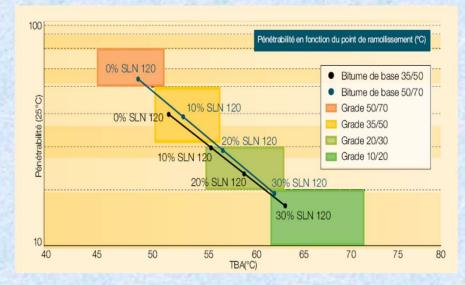
The **modifications** operate in such a way as to **increase** the **consistency**, **the viscosity** and the **stability** of the original bitumen

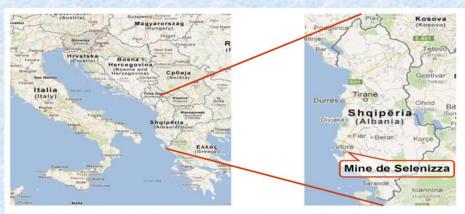


Characterisation of natural bitumen Selenizza®SLN

| Description | Penetration [dmm] | TR&B[°C] | Penetration Index | Grade obtained |
|--------------------------------|----------------------|----------|----------------------|----------------|
| Petroleum bitumen 50/70 | 54 | 49,0 | -1,28 | - |
| Mixed with 5% natural bitumen | 38 | 52,6 | -1,18 | 35/50 |
| Mixed with 10% natural bitumen | 28 | 56,2 | -1,01 | 20/30 |
| Mixed with 15% natural bitumen | 20 | -61,6 | -0,60 | 10/20 |
| Natural Bitumen | 0 | 120,0 | -0,18 | |

Structurally, the **organic phase** of Selenizza can be **compared** to **crude oil bitumen**, but with different **proportions** of maltenic and asphaltenic **fractions**, making it **100% compatible** with any type of road bitumen

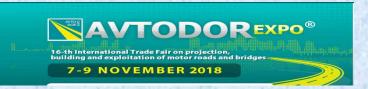






Characterization of natural bitumen Selenizza®SLN

- A PhD thesis was recently presented at the University of Strasbourg in France, on the potential of using natural bitumen in the production of hard penetration grade binders and high modulus asphalt mixes that lead to implementation of cost effective pavements (thin and long lasting pavement layers)
- ➤ The study, in line with the strategy of sustainable industrial development, proposes an alternative method using natural bitumen to produce HMA aging resistant and relatively efficient at low temperatures.
- ➤ These researches are very topical at this time, considering the problems encountered while using hard petroleum bitumen such as the risk of cold cracking, rapid aging, supply difficulties, as well as the inability to produce hard bitumen from certain crude oils...



IATROSCAN fractions

| | BARA BCAN method | Saturated [%] | Aromatic [%] | Resin %] | Asphaltene -i %] | I _c |
|--------------------------------|----------------------------------|---------------|--------------|--------------|-------------------------|----------------|
| Purified sample- depth | Average Standard deviation | 1,7 0,35 | 24,8 2,29 | 35,1 1,35 | 38,4 1,88 | 0,67 |
| Purified sample- surface | Average Standard deviation | 1,5 0,14 | 22,7 1,37 | 37,2 1,90 | 38,6 1,58 | 0,67 |
| Raw sample- depth | Average Standard deviation | 1,6 0,29 | 23,8 1,40 | 34,6 1,16 | 40,01 1,99 | 0,71 |
| Raw sample- surface | Average Standard deviation | 1,6 0,24 | 19,7 2,02 | 37,9 1,60 | 40,8 2,74 | 0,73 |

Evolution of glass transition temperatures

| | | Total heat flux | | | | | |
|-------------------------|----------|-----------------|-----------------------|----------------------|----------|--|--|
| | Tg1 [°C] | <u>T</u> g[°C] | T _g 2 [°C] | ΔT _E [°C] | ΔΦ [W/g] | | |
| Petroleum bitumen 50/70 | -31,9 | -22,9 | -13,2 | 18,6 | 0,022 | | |
| Mixed with 5% SLN | -30,9 | -23,1 | -13,8 | 17,1 | 0,019 | | |
| Mixed with 10% SLN | -30,3 | -23,1 | -13,3 | 17,0 | 0,018 | | |
| Mixed with 15% SLN | -32,1 | -23,3 | -13,4 | 18,8 | 0,019 | | |
| Natural asphaltite SLN | -12,6 | -1,1 | 16,2 | 28,8 | 0,021 | | |

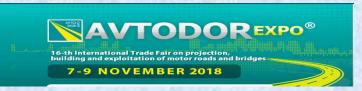
| Complex | Measures at 100°C, 5 Hz | | | | |
|-----------------------------|-------------------------|-------------|--|--|--|
| modulus | E* [GPa] | δ [°C] | | | |
| Albanian Natural Bitumen | 0,95 - 1,27 | 48,3 - 51,7 | | | |

SARA (IATROSCAN) fractions analysis, with colloidal instability index I_c values indicating a sol or sol-gel character.

Reach in resins and asphaltenes responsible for its elevated hardness, high R&BT, high |E*|, zero penetration

Compared to petroleum bitumen, Selenizza's organic phase has higher content of polar fractions (resin + aspahltene) resulting in a:

- vitreous transition at higher temperatures
- enhanced adhesion between the bitumen and mineral aggregates
- addition of natural bitumen does not affect the glass transition temperature of bitumen
- 35/50 compared to modified alternative → T_g = -23.1°C versus T_g = -19.3°C
- better resistance of natural bitumen to brittle fracture



Selenizza®SLN-Aging Inhibitor

RTFOT test (to simulate oxidation of bitumen during mixture manufacturing) & **PAV** (to simulate in-service ageing). Aging effect was quantified using the following mathematical expression:

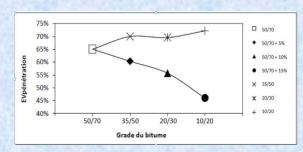
$$EV_x = \frac{|x^{RTFOT} + PAV - x^{New}|}{x^{New}} \cdot 100$$
 where EV_x - The evolution of the mechanical property x

1. **Changes** of modified specimens **were lower** than those of **50/70** 2. Changes are **attenuated** with the increase **of % SLN** 3. Modified bitumen are characterized by **minor changes** compared to petroleum bitumen of equivalent grades

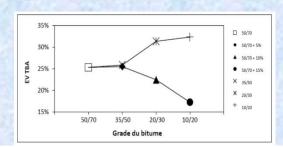
From the **ageing point of view**, the binders modified with Selenizza can **advantageously replace** hard petroleum bitumen for the production of HMAC

Evolution of Penetration after RTFOT and PAV ageing

| | P | Penetration (dmm) | | | | | TF | R&B (°C) | | |
|-----------------|---------------|-------------------|-----------------------|--------------|-----------------------|------------|----------------|-----------------------|--------------|-----------------------|
| Description | New binder | After RTFOT | Δ ₁ (%) | After PAV | Δ ₂ (%) | New binder | After RTFOT | Δ ₁ (%) | After PAV | Δ ₂ (%) |
| Petroleum 50/70 | 54 | 37 | 31.5 | 19 | 64.8 | 49 | 53.4 | 8.9 | 61.4 | 25.3 |
| Mixed with 5% | 38 | 27 | 28.9 | 15 | 60.5 | 52.6 | 57.2 | 8.7 | 66.0 | 25.4 |
| Mixed with 10% | 28 | 21 | 25 | 13 | 53.5 | 56.2 | 60.8 | 8.1 | 68.8 | 22.4 |
| Mixed with 15% | 20 | 14 | 30 | 11 | 45 | 61.6 | 65.4 | 6.1 | 72.2 | 17.2 |
| Petroleum 35/50 | 40 | 27 | 32.5 | 12 | 70 | 52.6 | 56.8 | 7.9 | 66.2 | 25.8 |
| Petroleum 20/30 | 23 | 12 | 47.8 | 7 | 69.5 | 60.0 | 67.0 | 11.6 | 78.8 | 31.3 |
| Petroleum 10/20 | 18 | 9 | 50 | 5 | 72.2 | 65.0 | 72.6 | 11.7 | 86.0 | 32.3 |



Evolution of R&B after RTFOT and PAV ageing

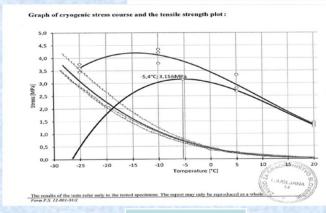




Low-temperature fracture

The Laboratory TSRST comparative tests from Slovenian National Building Institute (ZAG), conducted on base course asphalt mixture specimen AC 22 with 50/70 pen bitumen, and same mix design with 50/70 modified with SelenizzaSLN showed that Selenizza has little influence on the low temperature fracture resistance. SLN brings some rigidity (curves are shifted to the right, toward positive temperatures). At T= 3,3 °C, the asphalt mixture with Selenizza is subjected to a higher tensile strength 4,04 MPa, as compared with the tensile strength 3,156 MPa at T= -5,4 °C, for the classical asphalt mixture

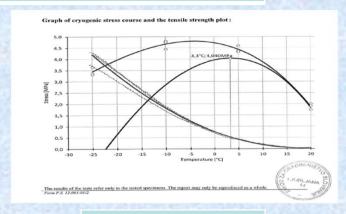
Graph of cryogenic stress course and the tensile strength plot



Binder 50/70

Test results summary:

| Failure temperature at TSRST | Tfailure | -28,4°C |
|--------------------------------------|---------------------------|----------|
| Maximum tensile strength reserve | $\Delta \beta_{t,max}$ | 3,156MPa |
| Temperature at $\Delta\beta_{t,max}$ | $T(\Delta \beta_{t,max})$ | -5,4°C |



Binder 50/70 + SLN

Test results summary:

| Failure temperature at TSRST | Tfailure | -25,3°C |
|--------------------------------------|---------------------------|----------|
| Maximum tensile strength reserve | $\Delta \beta_{t,max}$ | 4,040MPa |
| Temperature at $\Delta\beta_{t,max}$ | $T(\Delta \beta_{t,max})$ | 3,3°C |



In order to respond to the **technical challenges** imposed by:

- 1. very **severe stresses** and strains that bituminous pavements are subject to due to the **large increase** in the number **of trucks** crossing the Swiss Alps every year
- 2. **very harsh climatic** conditions of the country (temperatures fluctuate between -20°C and +40°C)

Switzerland has integrated in its national standard for bituminous mixtures, the concept of High Modulus Asphalt Mixes (EME)





- The performance class 1 is recommended to improve the resistance to permanent deformation (rutting)
- The class 2, to improve the fatigue resistance of the asphalt mix layer. More difficult to reach, it includes tough requirements on stiffness modulus and fatigue resistance (more severe than the French one):

Specifications of Swiss standard on EME

| Santagarda de la companya de la comp | Méthode d'essai | AC EME 22 C1 | AC EME 22 C2 |
|--|--------------------|-----------------|-----------------|
| Teneur en vides des éprouvettes Marshall (%) | EN 12697-8 | ≤ 3.0 - 5.0 | ≤1.0 - 3.0 |
| Sensibilité à l'eau, résistance à la traction par fendage ITSR (%) | EN 12697-12 | ≥ 70 | ≥ 70 |
| Teneur en liant en pourcentage de la masse d'enrobé (%) | | ≥ 4.6 | ≥ 5.4 |
| Résistance à l'orniérage à 30 000 cyles et 60 °C | EN 12697-22 | | |
| Profondeur d'ornière sur une plaque de 10 cm d'épaisseur (%) | Enimorn | ≤ 5.0 | ≤ 7.5 |
| Module complexe à 15 °C/10 Hz (MPa) | EN 12697-26 | ≥ 11 000 | ≥ 14 000 |
| Résistance à la fatigue à 10 °C/25 Hz (microdéformations) | EN 12697-24 | ≥ 100 | ≥ 135 |



To respond to this constraints, CO.MI.BIT, an asphalt mix manufacturing cooperative, located in Taverne (Canton Ticino), has developed a new mix design of type AC EME 22 C2 that improves the fatigue performance by using a polymer modified bitumen all while maintaining a high stiffness modulus using Selenizza®SLN as hardening additive. The binder was composed of a Shell Cariphalte 25 RC PmB and natural bitumen Selenizza SLN. Two alternatives of mix design same grading curve, have been tested a final binder with penetration ranging between 10 to 20 dmm

3.9% Shell Cariphalte 25 RC+ 1.4% SLN = 5.3% (Selenizza 26% of the total binder):

3.9% Shell Cariphalte 25 RC+ 1.6% SLN = 5.5% (Selenizza 29% of the total binder):

| Composition du liant | Unité | Formule 1 | Formule 2 |
|---|----------|-----------|------------|
| Shell Cariphalte 25 RC | % | 3,9 | 3,9 |
| SLN 120 Section we may be a supreme 1912 section 1 | % | 1,4 | 1,6 |
| Teneur en liant théorique (en % de la masse d'enrobé) | % | 5,3 | 5,5 |
| Module complexe à 15 °C/10 Hz (EN 12697-26) | MPa | 19 441 | 18 336 |
| Pourcentage de vides hydrostatique | % | | |
| Résistance à la fatigue à 10 °C/25 Hz (EN 12697-24) | Microdef | 139 | 145 |
| Pourcentage de vides hydrostatique | % | | Almost . A |

Tableau 4
Résultats des essais de module et de fatigue obtenus par Shell Global Solutions



- ➤ The obtained **modulus** and **fatigue** tests results **clearly exceed** the Swiss standard specification for the asphalt mixes AC EME 22 C2.To prevent the **cracking risk** at low temperatures, **a new job** mix formula was envisioned and then verified by **LAVOC Laboratory** at the Swiss Federal Institute of Technology Lausanne.
- The mix design was modified with a **less strong value** of the **stiffness** modulus by introducing a lower percentage of Selenizza (22%), while maintaining a **high level of fatigue resistance**:

4.7% Shell Cariphalte 25 RC+ 1.4% SLN = 6.1% Richness modulus K= 3.74

Test results conducted by LAVOC, was the following (void content= 2.4%):

 $\square \mathcal{E}_6$ (extrapoled) $\approx 150 \, \mu def$

(Swiss standard ≥135 µdef)

 \square Modulus (15°C/10 Hz) = 15 100 MPa

(Swiss standard ≥ 14 000 MPa)



Other mix design of type AC EME 22 C1 were developed and validated in cooperation with LAVOC Laboratory at the Swiss Federal Institute of Technology Lausanne, with very good results in terms of fatigue performance and with low susceptibility to rutting

3.9 % PmB Shell Cariphalte 25 RC + 1,4% SLN = 5.3%

Test results:

| ☐ Richness modulus K=3.30 | (standard ≥2.7) |
|---|-------------------------|
| ☐ Rutting (at 30 000 cycles) = 1.9 % | (standard ≤5%) |
| □ ε ₆ (extrapoled) ≈ 134 microdéformations | (standard ≥100 µdef) |
| ☐ Modulus (15°C/10 Hz) = 18 016 MP a | (standard ≥ 11 000 MPa) |

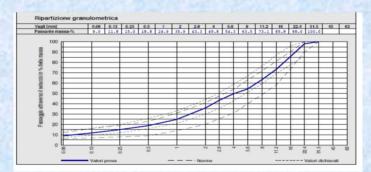
The high value of richness modulus generates good fatigue performances and the asphalt mix has low rutting susceptibility



➤ It should also be noted the use of Selenizza with the recycled aggregates

Example: AC EME 22 C2 with binder Shell B 15/20 + 10% RA + 0.3% SLN

Binder content = 5.34% (4.4% Shell 15/20 + 0.64%RA + 0.3% SLN)



- ☐ Rutting (at 30 000 cycles) = 3.8 %
- □ \mathcal{E}_6 (extrapoled) ≈ 153 microdéformations
- **☐** Modulus $(15^{\circ}C/10 \text{ Hz}) = 14 800 \text{ MPa}$

(standard ≤7.5%)

(standard ≥130 µdef)

(standard ≥ 14 000 Mpa)

The mixture has low rutting susceptibility and is effective in terms of fatigue resistance. The good behavior that was observed was also due to the addition of Selenizza



In Switzerland, after 10 years' service with harsh winters, in the various road construction projects where Selenizza was used for the manufacture of high performing EME, no thermal cracks were found confirming the relevance of these EME mix designs that have led to high mechanical performances

These results confirm the potential for successfully combining the benefits of polymer modified bitumen and of natural bitumen Selenizza SLN





2011: Bridge Val Verzaska, Ticino - Switzerland





Lugano - Switzerland





Mastic asphalt Switzerland



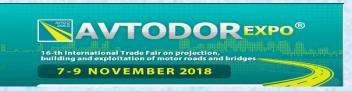


Highway Ticino - Switzerland



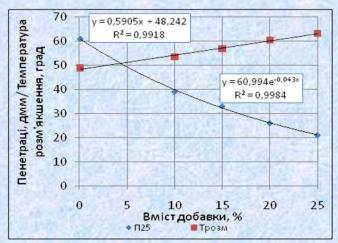


2011: Highway Bypass Bern - Switzerland



In Ukraine, SelenizzaSLN was is recognized as construction material and has been classified as a Bitumen modifier for asphalt mixes that should be inserted to a proportion of 4-12% by mass of the base Bitumen (mainly 5% and 8%).

The National Technical University of Ukraine has analyzed the properties of Selenizza aiming to determine its compatibility with the Ukrainian bitumen (60/90 grade bitumen) and with those of bordering foreign countries.

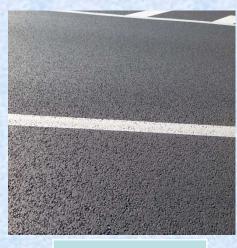


TR&B and Penetration Modification of BND 60/90 bitumen with the % of Selenizza



➤ Highway Kiev – Kovel, in the western part of Ukraine, surface paving SMA -20, 8cm base course & 8cm binder course, on 3 segments with a total length of approximately 5 km







with Selenizza

with polymers

After 3 years' service, the road is still in a very good condition !!!

In the middle of the highway constructed using Selenizza, on a small section, **for comparision purposes**, was used a **polymer modified binder**. The asphalt roadway surface with Selenizza is **smoother** and is more **black in color** compared to the segment where Kraton modified binder was used



Zhitomir, 70 Km west of Kiev



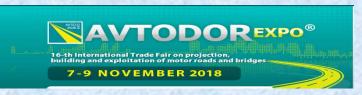


Before After





National highway Mykolaiv (Ukraine)





SMA with 6 % Selenizza





Heavy traffic highway interchange entering Kiev



SELENIZZA SLN an additive whose amount of percentage incorporated is deduced from the total quantity of the base bitumen

| | | BII | BINDER | |
|-----------------|------------|-------------------|-------------|--------------------------|
| EXAMPLE | AGGREGATES | Normal bitumen | S L N | Replaces |
| BASIC JMF | 95 % | 5% | | Ţ |
| ALTERNATIVE JMF | 95 % | 4% | 1% | 1 % Normal bitumen |



LIFE CYCLE ASSESSMENT SELENIZZA compared to petroleum bitumen

Consumers and Goverments are increasingly demanding information about the sustainability of products and interested in comparing potential solutions. Life-cycle Assessment (LCA) focuses on the sustainability of different pavements and materials, seeking to measure the environmental impacts of all stages of a product's life, from cradle to grave

University of Rome in cooperation with the company Selenice Bitumi, carried out e research project, whose aim was to analyze and compare for the first time, in terms of energy consumption and CO₂ emission between:

- 1. the various steps necessary to produce the **conventional bitumen from crude oil**
- 2. the production process of the **Albanian natural asphalt** (Selenizza)



LIFE CYCLE ASSESSMENT SELENIZZA compared to petroleum bitumen

Deposit of natural bitumen Selenizza







LIFE CYCLE ASSESSMENT SELENIZZA compared to petroleum bitumer

Comparing the results

| 100 | Bitume | s routiers de d | stillation | | |
|--------------------------------------|----------------|------------------|------------|------|--------|
| Total Consommation d'énergie | MJ/t | | | | 4,71 |
| CO ₂ Emissions dans l'air | g | 144563 | 37422 | 7831 | 226167 |
| | | | • | | |
| | L'asph | nalte naturel Se | elenizza | | |
| Total Consommation d'énergie | L'asph MJ/t | nalte naturel Se | elenizza | | 2,376 |

- Selenizza's production cycle has an environmental impact approximately 44% less than the distillation bitumen
- Energy consumption is also reduced by around 50% compared to bitumen produced from crude oil



Main outcomes

The aging effect and thermal fatigue are coupled. Low temperature fatigue cracking and ageing, are cumulative in effect by amplifying the thermal stress generated during cooling. Consequently, it is reasonable to assume that the use of binders modified with Selenizza, offers an advantage in terms of behavior of asphalt mixes at low temperatures compared with equivalent petroleum paving grade bitumen

The **combination** of **natural bitumen** with **polymers** offers an **attractive solution** on a **technical** and on an **economic level**. **A part of SBS** may **be replaced** by natural bitumen in order to **reduce the cost** of asphalt mixture production. Also, according to some authors, the replacement of SBS by natural asphalt, **increases the workability**, which could allow the reduction of the **compaction energy**.

The addition of natural bitumen in polymer modified bitumen, leads to an increase of the magnitude of the complex modulus and of the phase angle. This interesting result, shows that the combination of natural bitumen with polymers, simultaneously improves the binder rigidity, which is responsible for the good performance at high temperatures, with its viscous dissipation, that is responsible for low temperature performance.



Conclusions

- ➤ 100% compatible with bitumen from refinery (and polymer modified bitumen)
- > High performance in modulus & permanent deformation
- Better bitumen-aggregates adhesion
- Pavement thickness reduction
- Better workability
- > Aging retarder & Higher lifetime of the pavements
- Minor environmental impact





Дякую за увагу!

